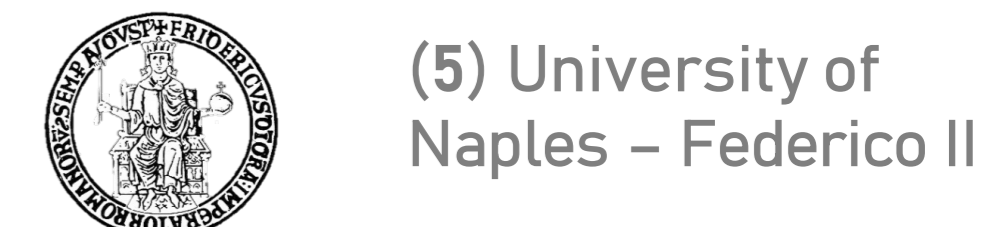
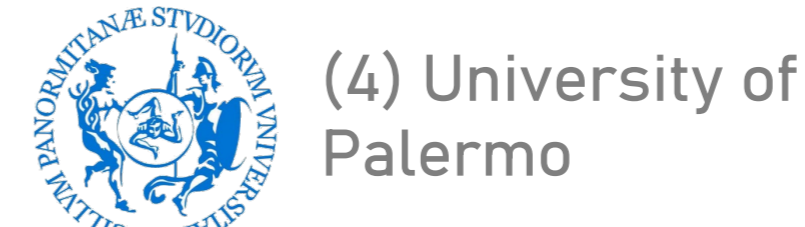
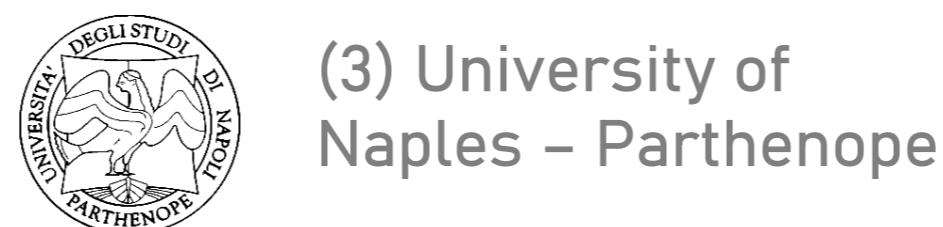
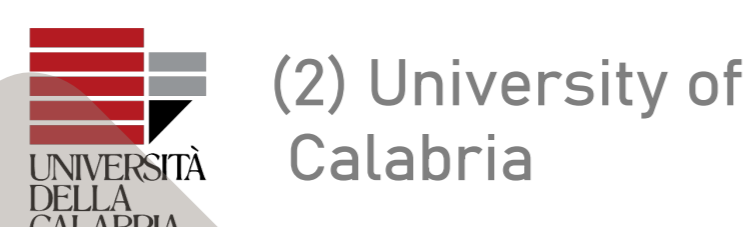


# Application of FRCM composites to strength masonry structural elements

M.A. Aiello<sup>1</sup>, A. Cascardi<sup>2</sup>, F. Ceroni<sup>3</sup>, L. La Mendola<sup>4</sup>, G.P. Lignola<sup>5</sup>, M. C. Oddo<sup>4</sup>, A. Prota<sup>5</sup>



## FRCM: ADVANTAGES and OPPORTUNITIES

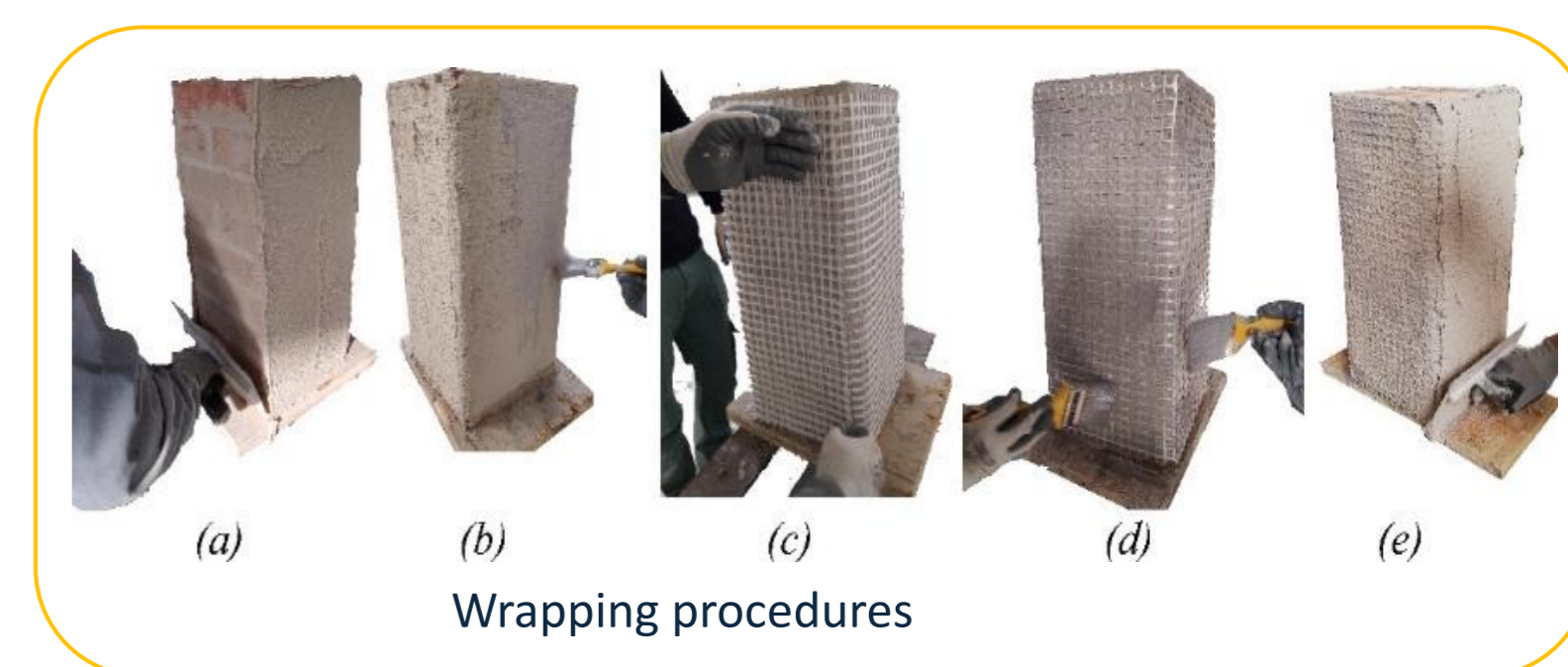
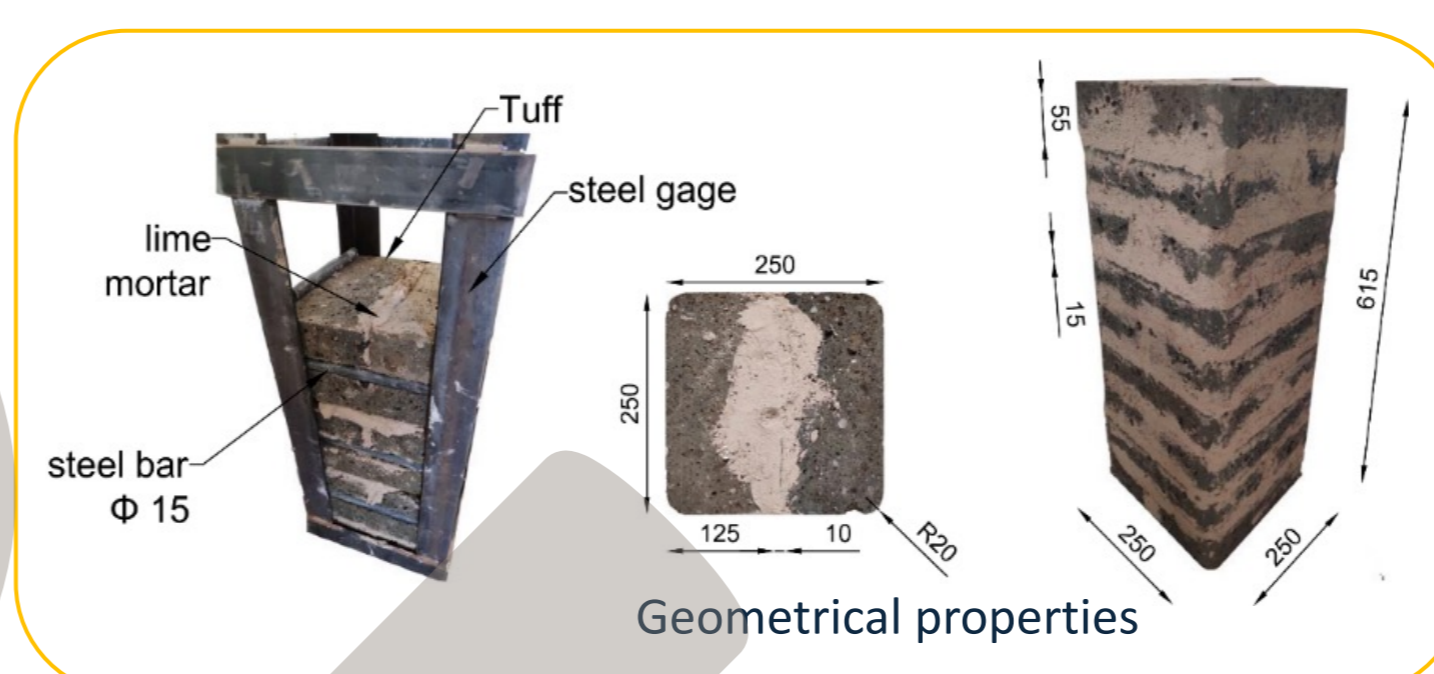
- ✓ Sustainability
- ✓ Compatibility with the substrate
- ✓ Reversibility of interventions
- ✓ Resistance to relatively high temperatures
- ✓ No toxic exposures

Generally, a single ply of external reinforcement produced a negligible increase of bearing capacity, while the same strengthening systems applied with multi-ply strengthening schemes produced a significant increase in terms of strength and ultimate axial deformation.

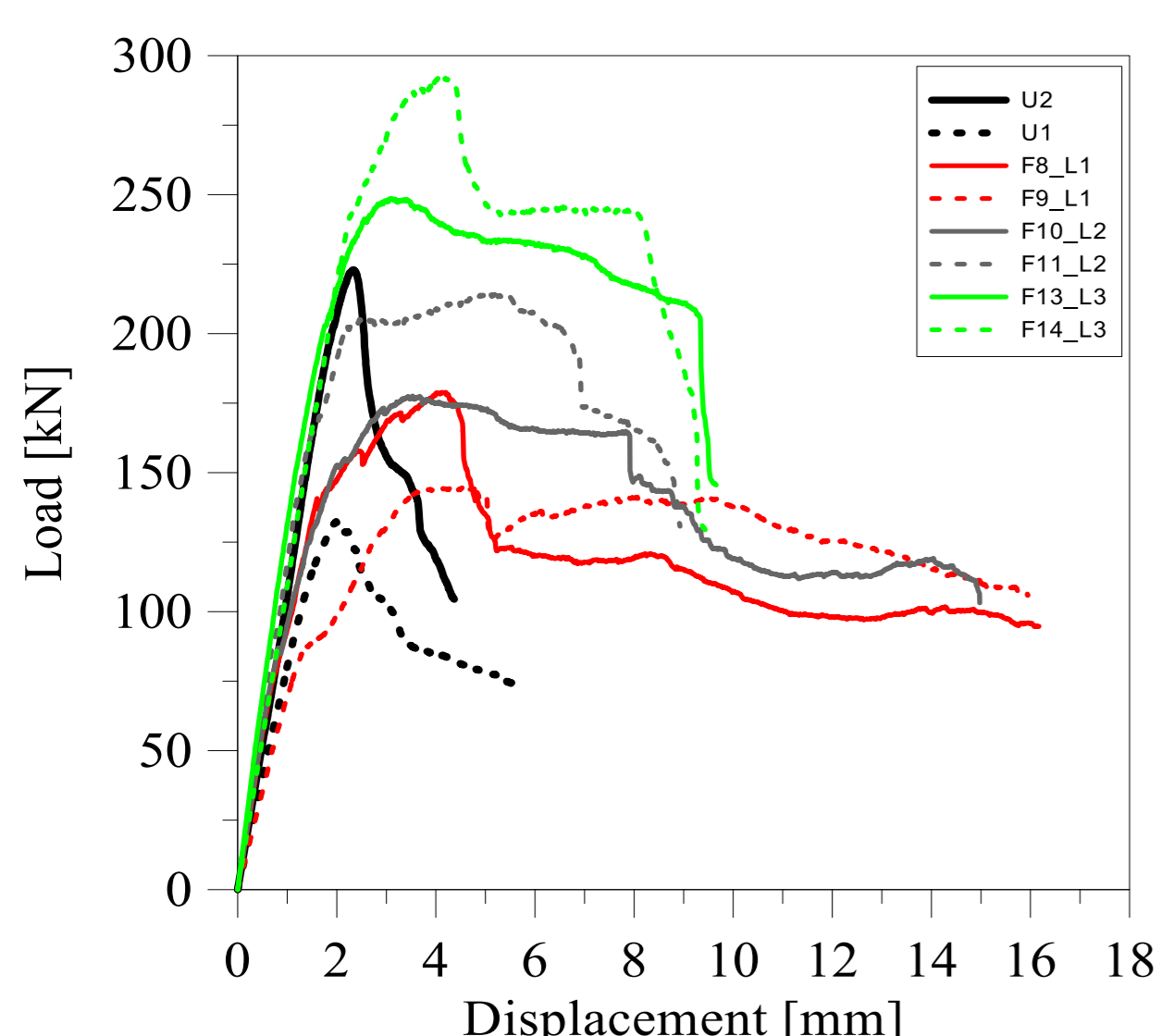
## Confinement of masonry columns with FRCM

The use of FRCM composites is a highly effective technique for enhancing the strength and shortening capacity of masonry columns subjected to axial load. Results from experimental tests on small scale confined masonry columns provide valuable insights into the benefits of FRCM systems.

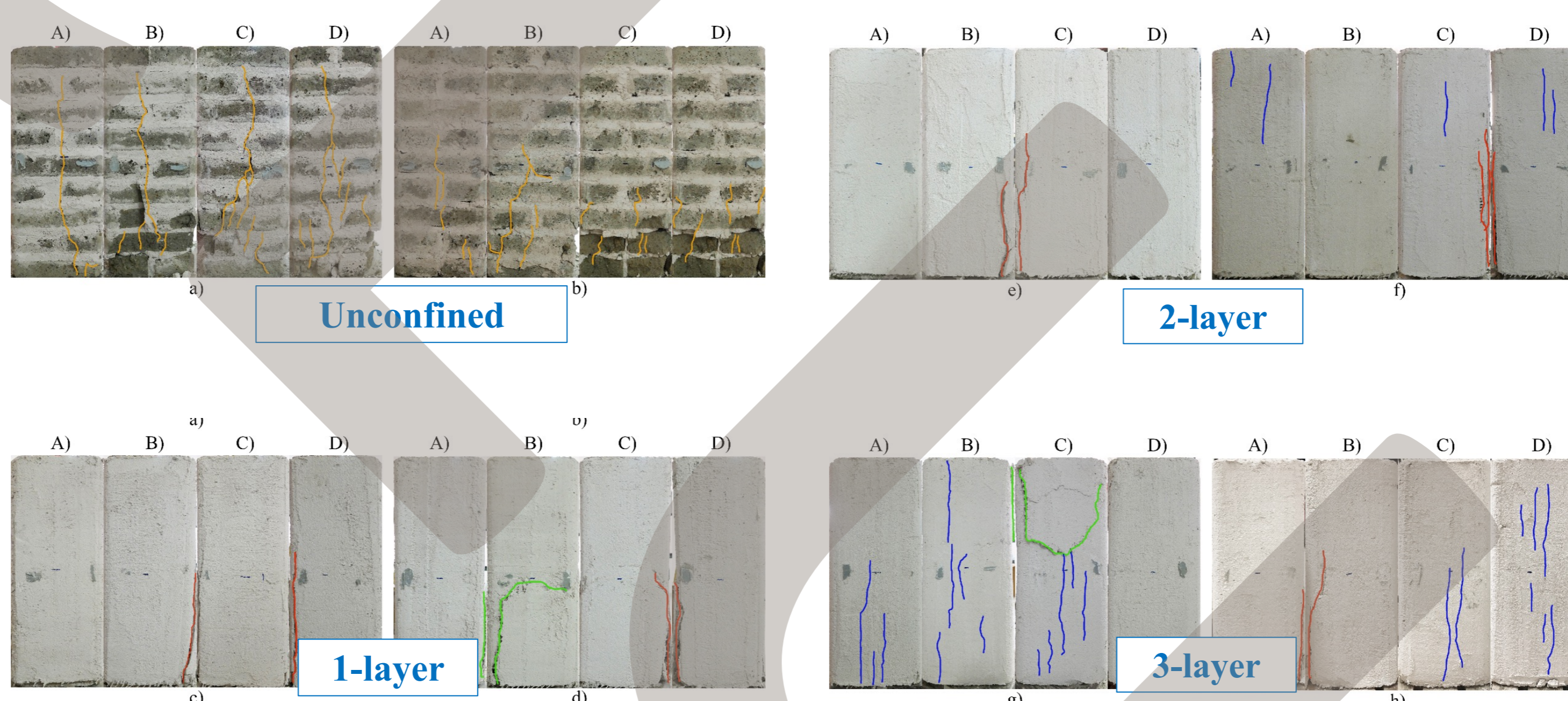
### EXPERIMENTAL CAMPAIGN ON TUFF MASONRY COLUMNS CONFINED WITH GLASS-FRCM



## LOAD-DISPLACEMENT CURVE



## FAILURE MODES



**Crushing (C):** longitudinal cracking of the masonry

**Knife-effect (KE):** fiber rupture at the corner of the cross section

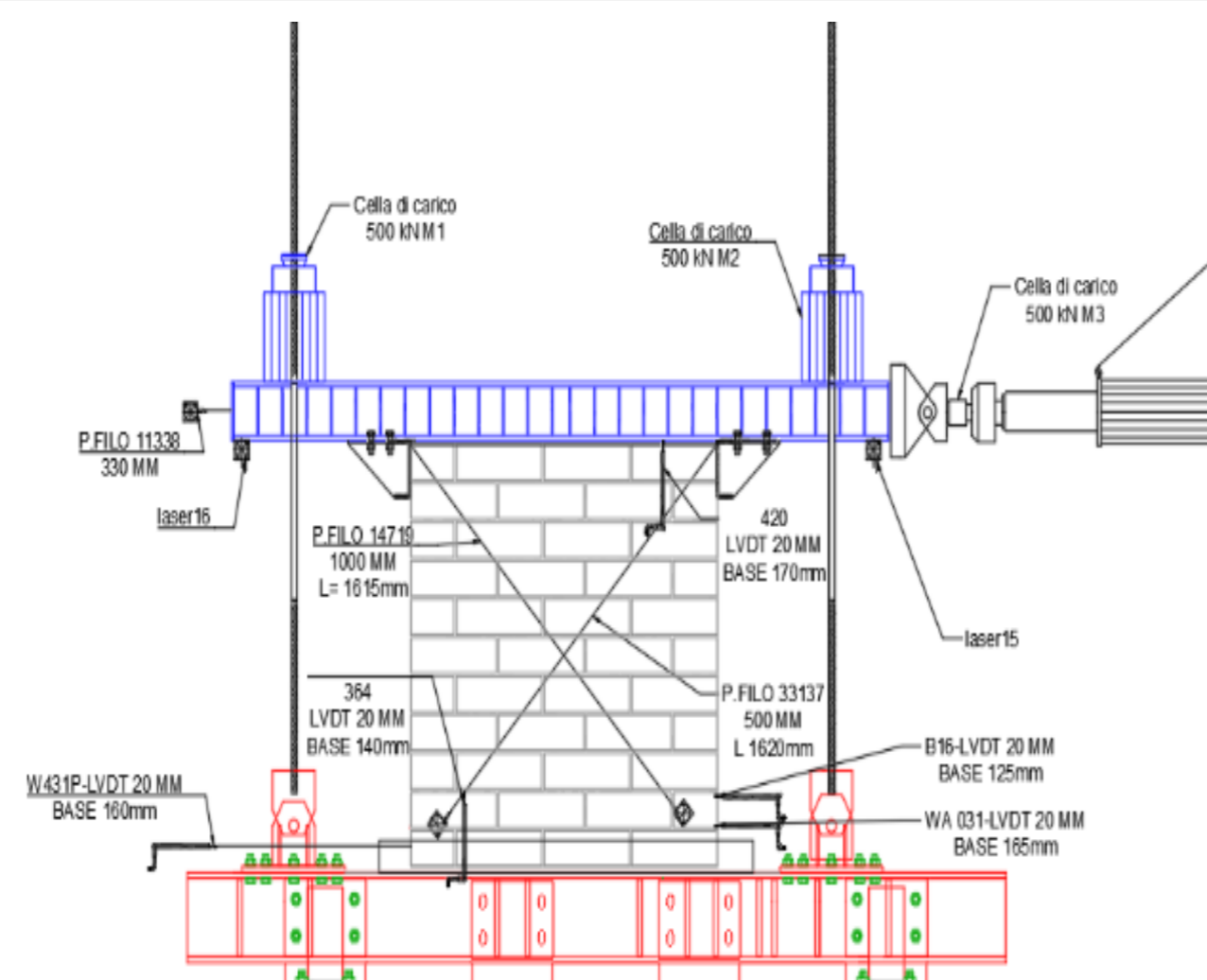
**Matrix Cracking (MC):** longitudinal cracking of the FRCM lime-based mortar

**Detachment (D):** separation of the external layer of the FRCM-matrix

## Shear compression tests

Shear compression tests performed on masonry tuff panels with dimension 1.3×1.3×0.25 m strengthened with a basalt FRCM bidirectional grid and hydraulic lime (mortar thickness about 10 mm). Same materials (tuff stones, mortar, FRCM) used in bond tests.

### EXPERIMENTAL SET-UP FOR SHEAR COMPRESSION TESTS



Reproduction of a pier panel (wall) with a rigid spandrel on top (no rotation on top).

Vertical load of 200 kN, compression stress  $\approx 0.60\text{MPa}$  ( $\nu=0.23$ ).

## RESULTS

### Unstrengthened panels (URM)

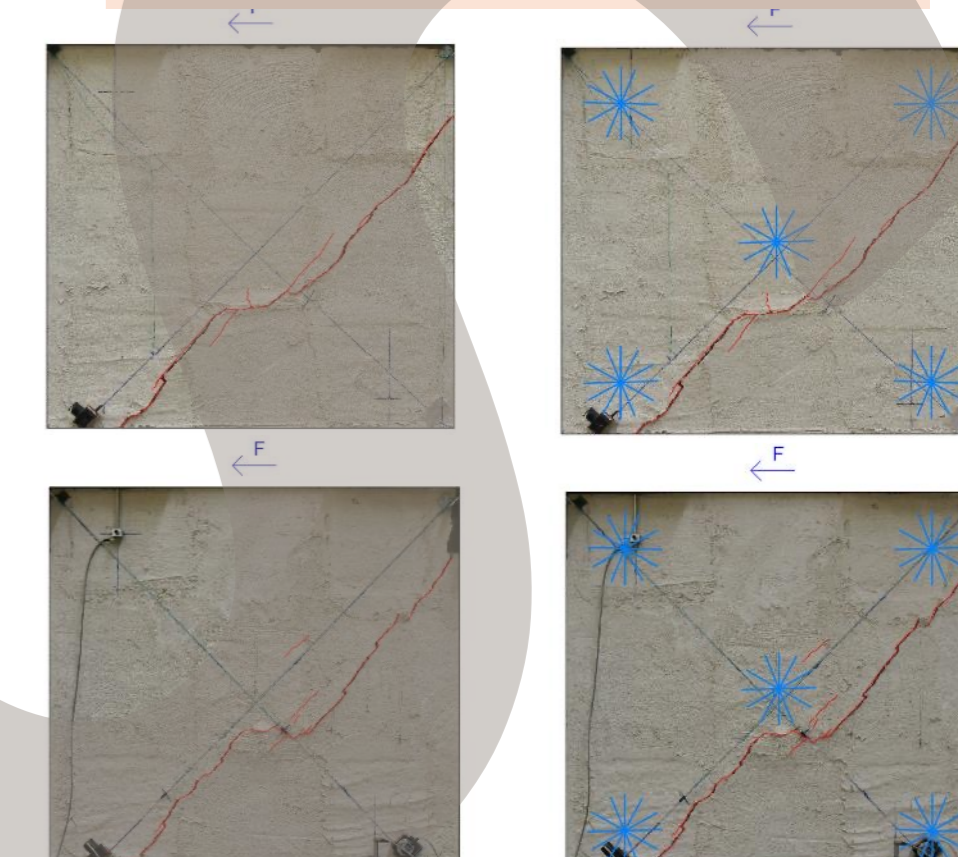
Average  $F_{max} = 116,7\text{ kN}$



Shear Failure in tuff blocks

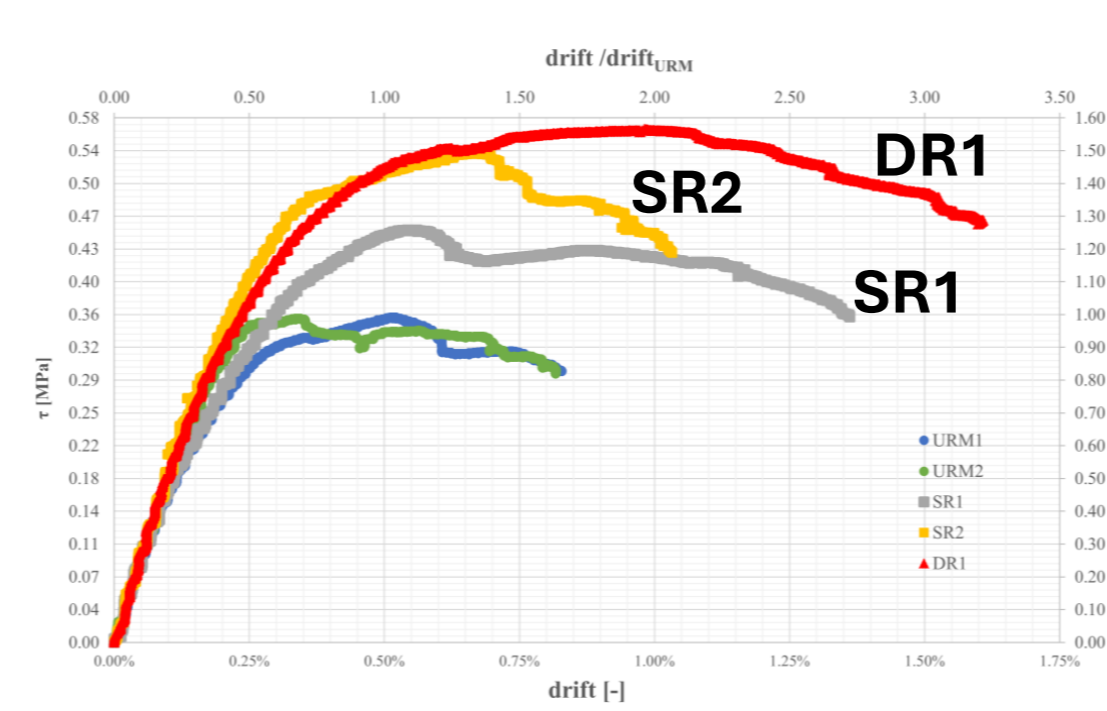
### 1-side Reinforced panels (SRM)

Average  $F_{max} = 162,1\text{ kN}$



Shear Failure in tuff blocks

✓ Shear resistance of masonry panels: effectiveness of reinforcement



### Single-side reinforced panels (SRM)

$F_{max,average} = 162,1\text{ kN}$

+ 39% with respect to URM

- Comparable stiffness
- Relevant influence of FRCM on strength

### Double-side reinforced panel (DRM)

$F_{max} = 184,0\text{ kN}$  (flexural failure of the wall)

+ 58% with respect to URM

+ 14% with respect to SR